

## **GENERAL INFORMATION**

# **CONCRETE HANGERMATE®+**

Rod Hanging Anchor

## PRODUCT DESCRIPTION

The Hangermate®+ concrete screw is a one piece, steel anchor designed for rod hanging applications such as fire protection systems, ventilation systems, electrical conduit, pipe hanging and cable trays. Tested and qualified for use in cracked concrete and seismic conditions. The concrete Hangermate®+ requires ANSI masonry bits for installation, accepts 1/4", 3/8" or 1/2" diameter threaded rods. It is also available in a 3/8" male thread version.

#### **GENERAL APPLICATIONS AND USES**

•	Fire	Sprinkler	<b>Pipes</b>
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- · Ventilation Systems
- Cable Trays
- Lighting Systems

•	Sus	pended	l Ceilings
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- Overhead Utilities
- Tension zone / cracked concrete
- Seismic qualification (SDC A − F)

#### **FEATURES AND BENEFITS**

- + Installs into holes drilled with a standard ANSI drill bit
- + Fast installation with power tools resulting in labor savings
- + Patented thread design offers low installation torque
- + Tough threads for tapping high strength concrete

#### **APPROVALS AND LISTINGS**

- International Code Council, Evaluation Service (ICC-ES), ESR-3889 for concrete; code compliant with the International Building Code/International Residential Code: 2024 IBC/IRC, 2021 IBC/IRC, 2018 IBC/IRC and 2015 IBC/IRC.
- Tested in accordance with ACI 355.2/ASTM E488 and ICC-ES AC193 for use in cracked and uncracked concrete and for use with the design provisions of ACI 318 (-19 and -14) Chapter 17 or ACI 318-11 Appendix D.
- Evaluated and qualified by an accredited independent testing laboratory for recognition in cracked and uncracked concrete including seismic and wind loading (anchor category 1)
- Evaluated and qualified by an accredited independent testing laboratory for reliability against brittle failure, e.g. hydrogen embrittlement.
- FM Approvals (Factory Mutual) see approval for sizes.
- City of Los Angeles, LABC Supplement (within ESR-3889)
- Florida Building Code, FBC Supplement including HVHZ (within ESR-3889)

#### **GUIDE SPECIFICATIONS**

CSI Divisions: 03 16 00 - Concrete Anchors, 05 05 19 - Post-Installed Concrete Anchors. Anchors shall be Concrete Hangermate+ as supplied by DEWALT, Towson, MD. Anchors shall be installed in accordance with published instruction and the Authority Having Jurisdiction.

# **MATERIAL SPECIFICATIONS**

Anchor component	Specification
Anchor Body	Case hardened low carbon steel
Plating	Zinc plating according to ASTM B633, SC1 Type III (Fe/Zn 5) Minimum plating requirements for Mild Service Condition.

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CONCRETE HANGERMATE+ (INTERNALLY THREADED COUPLER HEAD)



CONCRETE HANGERMATE+ (EXTERNAL THREAD - STUD HEAD)

#### **THREAD VERSION**

• Unified Coarse Thread (UNC)

#### **ANCHOR MATERIALS**

· Zinc Plated Carbon Steel

#### **ANCHOR SIZE RANGE (TYP.)**

• 1/4", 3/8", and 1/2" diameter (threaded heads)

#### **SUITABLE BASE MATERIALS**

- · Normal-weight concrete
- · Lightweight concrete
- · Concrete over steel deck
- Hollow core concrete











#### **INSTALLATION SPECIFICATIONS**

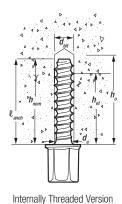
#### Installation Specifications for Hangermate+ in Concrete and Supplementary Information<sup>1,2</sup>

A		Notation	11.21.	Nominal Anchor Diameter (inch)							
Anchor Prop	Anchor Property/Setting Information		Units	1/4	3	/8	3.	/8	3/8	1/2	
Coupler thread s	size (UNC)	-	in.	1/4-20	3/8	3-16	3/8	-16	3/8-16	1/2-13	
Coupler head sty	yle	-	-	Internal Thread	Interna	l Thread	External Thread		Internal Thread	Internal Thread	
Nominal anchor (screw anchor be		da	in. (mm)	0.250 (6.4)		0.250 (6.4)		250 .4)	0.375 (9.5)	0.375 (9.5)	
Nominal drill bit	diameter (ANSI)	dbit	in.	1/4	1.	/4	1.	/4	3/8	3/8	
Minimum nomin	al embedment depth⁴	h <sub>nom</sub>	in. (mm)	1-5/8 (41)	1-5/8 (41)	2-1/2 (64)	1-5/8 (41)	2-1/2 (64)	2 (51)	2 (51)	
Effective embedment		hef	in. (mm)	1.20 (30)	1.20 (30)	1.94 (49)	1.20 (30)	1.94 (49)	1.33 (33)	1.33 (33)	
Minimum hole depth		h₀	in. (mm)	2 (51)	2 (51)	2-7/8 (73)	2 (51)	2-7/8 (73)	2-3/8 (60)	2-3/8 (60)	
Minimum concrete member thickness		h <sub>min</sub>	in. (mm)	3-1/4 (83)	3-1/4 (83)	4 (102)	3-1/4 (83)	4 (102)	3-1/2 (89)	3-1/2 (89)	
Minimum edge distance <sup>3</sup>		Cmin	in. (mm)	1-1/2 (38)	1-1/2 (38)		1-1/2 (38)		$c_{min} = 1-1/2 (38)$ for $s_{min} \ge 3 (76)$ ;	$c_{min} = 1-1/2 (38)$ for $s_{min} \ge 3 (76)$ ;	
Minimum spacin	Minimum spacing distance <sup>3</sup>		in. (mm)	1-1/2 (38)	1-1/2 (38)		1-1/2 (38)		$s_{min} = 2 (51)$ for $c_{min} \ge 2 (51)$	$s_{min} = 2 (51)$ for $c_{min} \ge 2 (51)$	
Nominal anchor	length <sup>6</sup>	lanch	in.	1-5/8	1-5/8	2-1/2	1-5/8	2-1/2	2	2	
Maximum impac	ct wrench power (torque) <sup>1</sup>	T <sub>impact,max</sub>	ftlbf. (N-m)	150 (203)	150 (203)		150 (203)		300 (407)	300 (407)	
Maximum manu	al installation torque	T <sub>inst,max</sub>	ftlbf. (N-m)	19 <sup>[3]</sup> (26)	19 <sup>[3]</sup> (26)	25 (34)	19 <sup>[3]</sup> (26)	25 (34)	25 (34)	25 (34)	
	Wrench socket size	-	in.	3/8	1.	/2	1.	/2	1/2	11/16	
Coupler Head	Max. head height	-	in.	33/64	43	/64	1-3	/16	43/64	53/64	
	Max. washer diameter	-	in.	1/2	21	/32	21.	/32	21/32	31/32	
Effective tensile (screw anchor be		Ase	in.² (mm²)	0.045 (28.8)		)45 3.8)		)45 3.8)	0.094 (60.7)	0.094 (60.7)	
Minimum specifi	ied ultimate strength	f <sub>uta</sub>	psi (N/mm²)	115,000 (793)		,000 93)	115 (79	,000 93)	100,000 (690)	100,000 (690)	
Minimum specifi	ied yield strength	fy	psi (N/mm²)	92,000 (634)		000 34)	92, (63		80,000 (552)	80,000 (552)	
Mean axial	Uncracked concrete	$eta_{ ext{uncr}}$	lbf/in.	1,381,000	1,38	1,000	1,38	1,000	1,157,000	1,157,000	
Stiffness <sup>7</sup>	Cracked concrete	$eta_{ ext{cr}}$	lbf/in.	318,000	318	,000	318	,000	330,000	330,000	

For SI: 1 inch = 25.4 mm, 1 ft-lb = 1.356 N-m, 1 psi = 0.0069 N/mm<sup>2</sup> (MPa).

- 1. The information presented in this table is used in conjunction with the design criteria of ACI 318 (-19 or -14) Chapter 17 or ACI 318-11 Appendix D, as applicable.
- 2. For installations through the soffit of steel deck assemblies into concrete, see the design information table for installation in the soffit of concrete-filled steel deck assemblies and the installation details in the soffit of concrete over steel deck for the applicable steel deck profile.
- 3. For installations into lightweight concrete, the max installation torque, Tinst,max, is 18 ft.-lb for nominal 1/4-inch-diameter anchors (screw anchor body diameter) with an 1-5/8-inch nominal embedment.
- 4. The embedment depth, h<sub>nom</sub>, is measured from the outside surface of the concrete member to the embedded end of the anchor.
- 5. Additional combinations for minimum edge distance, cmin, and minimum spacing distance, smin, may be derived by linear interpolation between the given boundary values for the nominal 3/8-inch-diameter anchors (screw anchor body diameter).
- The listed anchor length is based on coupler head anchor sizes commercially available at the time of publication compared with the requirements to achieve the minimum nominal embedment depth. The nominal anchor length is measured from under the coupler head to the tip of the anchor
- 7. Mean values shown, actual stiffness varies considerably depending on concrete strength, loading and geometry of application.

#### **Hangermate+ Anchor Detail in Concrete**

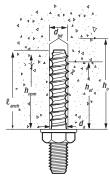


#### Nomenclature

Anchor diameter (screw anchor body)
Diameter of Drill Bit dbit

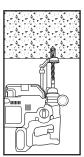
Minimum Nominal Embedment h<sub>nom</sub>

Effective Embedment hef Minimum Hole Depth h₀ Nominal Anchor Length

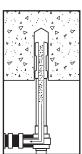




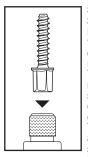
## **INSTALLATION INSTRUCTIONS**



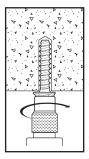
#### Step 1 Using the proper drill bit size, drill a hole into the hase material to the required depth. The tolerances of the drill bit used should meet the requirements of ANSI Standard B212.15.



Step 2 Remove dust and debris from hole during drilling (e.g. dust extractor, hollow bit) or following drilling (e.g. suction, forced air) to extract loose particles created during drilling.

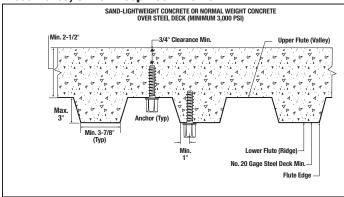


#### Step 3 Select a powered impact wrench or torque wrench and do not exceed the maximum torque. Timpact,max or Tinst,max, repectively, for the selected anchor diameter and embedment (See Table 1). Attach an appropriate sized hex socket to the wrench. Mount the screw anchor head into the socket.



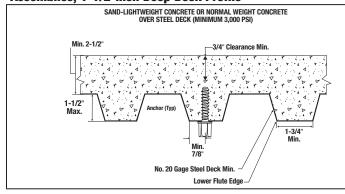
Step 4 Drive the anchor with an impact wrench or torque wrench through the fixture and into the hole until the head of the anchor comes into contact with the member surface. Do not spin the hex socket off the anchor to disengage. Insert threaded rod or threaded bolt element into Hangermate+.

# Hangermate+ Installation Detail for Screw Anchors in the Soffit of Concrete over Steel Deck Floor and Roof Assemblies, 3-inch Deep Deck Profile<sup>1,2,3</sup>



- Anchors may be placed in the upper flute or lower flute of the concretefilled steel deck profiles provided the minimum hole clearance of 3/4-inch is satisfied for the selected anchor. See the Tension and Shear Design information for Anchors Installed in the Soffit of Concrete-Filled Steel Deck Assemblies table.
- 2. Anchors in the lower flute may be installed with a maximum 15/16-inch offset in either directions from the center of the flute. The offset distance may be increased proportionally for profiles with lower flute widths greater than those shown provided the minimum lower flute edge distance is also satisfied (e.g. 1-1/4-inch offset for 4-1/2-inch wide flute).
- 3. See the Tension and Shear Design information for Anchors Installed in the Soffit of Concrete-Filled Steel Deck Assemblies table for design data.

## Hangermate+ Installation Detail for Screw Anchors in the Soffit of Concrete over Steel Deck Floor and Roof Assemblies, 1-1/2-inch Deep Deck Profile<sup>1,2,3</sup>



- 1. Anchors may be placed in the upper flute or lower flute of the concretefilled steel deck profiles provided the minimum hole clearance of 3/4-inch is satisfied for the selected anchor. See the Tension and Shear Design information for Anchors Installed in the Soffit of Concrete-Filled Steel Deck
- 2. Anchors in the lower flute may be installed in the center of the flute. An offset distance may be given proportionally for profiles with flute widths greater than those shown provided the minimum lower flute edge distance is also satisfied.
- 3. See the Tension and Shear Design information for Anchors Installed in the Soffit of Concrete-Filled Steel Deck Assemblies table for design data.



## **PERFORMANCE DATA (ASD)**

#### Ultimate Load Capacities for Hangermate+ in Normal-Weight Concrete<sup>1,2,3</sup>

	Nominal	Minimum	Minimum Concrete Compressive Strength									
Nominal Anchor	Anchor Diameter	Nominal Embedment	f'c = 2, (17.3	500 psi MPa)		000 psi MPa)		000 psi MPa)		000 psi MPa)	f'c = 8, (55.2	
Size (:	(screw anchor body) in.	Depth in. (mm)	Tension lbs (kN)	Shear lbs (kN)	Tension lbs (kN)	Shear Ibs (kN)	Tension lbs (kN)	Shear lbs (kN)	Tension lbs (kN)	Shear Ibs (kN)	Tension lbs (kN)	Shear lbs (kN)
1/4	1/4	1-5/8 (41)	2,410 (10.7)	1,485 (6.6)	2,545 (11.3)	1,525 (6.8)	2,775 (12.3)	1,525 (6.8)	2,775 (12.3)	1,525 (6.8)	2,775 (12.3)	1,525 (6.8)
3/8	1/4	1-5/8 (41)	2,410 (10.7)	1,555 (6.9)	2,545 (11.3)	1,565 (7.0)	2,775 (12.3)	1,565 (7.0)	2,775 (12.3)	1,565 (7.0)	2,775 (12.3)	1,565 (7.0)
3/0	1/4	2-1/2 (64)	3,650 (16.2)	1,555 (6.9)	3,855 (17.1)	1,565 (7.0)	4,200 (18.7)	1,565 (7.0)	4,270 (19.0)	1,565 (7.0)	4,270 (19.0)	1,565 (7.0)
3/8	3/8	2 (51)	3,670 (16.3)	1,985 (8.8)	4,020 (17.9)	2,010 (8.9)	4,645 (20.7)	2,010 (8.9)	4,725 (21.0)	2,010 (8.9)	5,455 (24.3)	2,010 (8.9)
1/2	3/8	2 (51)	3,670 (16.3)	2,970 (13.2)	4,020 (17.9)	2,990 (13.3)	4,645 (20.7)	2,990 (13.3)	4,725 (21.0)	2,990 (13.3)	5,455 (24.3)	2,990 (13.3)

- 1. Tabulated load values are for anchors installed in uncracked concrete. Concrete compressive strength must be at a minimum at the time of installation.
- 2. Ultimate load capacities must be reduced by a minimum safety factor of 4.0 or greater to determine allowable working load.
- 3. The tabulated capacities are for the Hangermate+ anchors which must be checked against the steel strength of the corresponding threaded rod or bolt size and type; the lowest load level controls.

## Allowable Load Capacities for Hangermate+ in Normal-Weight Concrete<sup>1,2,3,4,5,6</sup>



Nominal	Nominal	Minimum				Minimum Concrete Compressive Strength							
Nominal Anchor	Anchor Diameter	Nominal Embedment		500 psi MPa)		000 psi MPa)		000 psi MPa)		000 psi MPa)	f'c = 8, (55.2	000 psi MPa)	
Size in.	(screw anchor body) in.	Depth in. (mm)	Tension lbs (kN)	Shear Ibs (kN)	Tension lbs (kN)	Shear Ibs (kN)	Tension lbs (kN)	Shear lbs (kN)	Tension lbs (kN)	Shear Ibs (kN)	Tension lbs (kN)	Shear lbs (kN)	
1/4	1/4	1-5/8 (41)	605 (2.7)	370 (1.6)	635 (2.8)	380 (1.7)	695 (3.1)	380 (1.7)	695 (3.1)	380 (1.7)	695 (3.1)	380 (1.7)	
3/8	1/4	1-5/8 (41)	605 (2.7)	390 (1.7)	635 (2.8)	390 (1.7)	695 (3.1)	390 (1.7)	695 (3.1)	390 (1.7)	695 (3.1)	390 (1.7)	
3/0	1/4	2-1/2 (64)	915 (4.1)	390 (1.7)	965 (4.3)	390 (1.7)	1,050 (4.7)	390 (1.7)	1,070 (4.8)	390 (1.7)	1,070 (4.8)	390 (1.7)	
3/8	3/8	2 (51)	920 (4.1)	495 (2.2)	1,005 (4.5)	505 (2.2)	1,160 (5.2)	505 (2.2)	1,180 (5.2)	505 (2.2)	1,365 (6.1)	505 (2.2)	
1/2	3/8	2 (51)	920 (4.1)	745 (3.3)	1,005 (4.5)	750 (3.3)	1,160 (5.2)	750 (3.3)	1,180 (5.2)	750 (3.3)	1,365 (6.1)	750 (3.3)	

- 1. Tabulated load values are for anchors installed in uncracked concrete. Concrete compressive strength must be at the specified minimum at the time of installation.
- 2. Allowable load capacities are calculated using an applied safety factor 4.0.
- 3. Allowable load capacities must be multiplied by reduction factors when anchor spacing or edge distances are less than critical distances.
- 4. Linear interpolation may be used to determine allowable loads for intermediate embedments and compressive strengths.
- 5. For lightweight concrete, multiply the tabulated allowable load capacities by a reduction factor of 0.60.
- 6. The tabulated capacities are for the Hangermate+ anchors which must be checked against the steel strength of the corresponding threaded rod or bolt size and type; the lowest load level controls.

#### Allowable Load Capacities for Hangermate+ in Hollow-Core Concrete<sup>1,2,3,4,5,6,7</sup>



	Nominal	Minimum Nominal Embedment Depth in. (mm)	Minimum Concrete Compressive Strength								
Nominal Anchor	Anchor Diameter		f'c = 5,000 psi (34.5 MPa)		f <sup>i</sup> c = 6, (41.4	000 psi MPa)	f'c = 8,000 psi (55.2 MPa)				
	body)		Tension lbs (kN)	Shear Ibs (kN)	Tension lbs (kN)	Shear Ibs (kN)	Tension lbs (kN)	Shear Ibs (kN)			
1/4	1/4	1-1/2 (41)	695 (3.1)	380 (1.7)	695 (3.1)	380 (1.7)	695 (3.1)	380 (1.7)			
3/8	1/4	1-1/2 (41)	695 (3.1)	390 (1.7)	695 (3.1)	390 (1.7)	695 (3.1)	390 (1.7)			

- 1. Tabulated load values are for anchors installed in uncracked concrete. Concrete compressive strength must be at the specified minimum at the time of installation.
- 2. Allowable load capacities are calculated using an applied safety factor 4.0.
- 3. Allowable load capacities must be multiplied by reduction factors when anchor spacing or edge distances are less than critical distances.
- 4. Linear interpolation may be used to determine allowable loads for intermediate embedments and compressive strengths.
- 5. Hollow core concrete must have a minimum cover thickness of 1-1/2" below the core locations. where anchors are installed. Care must be taken to prevent damage to prestressed cables in the hollow core concrete panel during drilling and installation.
- 6. Tabulated capacties are for PFM2211100, PFM2211200 and PFM1421000 Hangermate+, as applicable.
- 7. The tabulated capacities are for the Hangermate+ anchors which must be checked against the steel strength of the corresponding threaded rod or bolt size and type; the lowest load level controls



#### LOAD ADJUSTMENT FACTORS FOR NORMAL-WEIGHT CONCRETE

# Edge Distance Reduction Factors - Tension ( $F_{\text{NC}}$ )

Nomin	al Anchor Size (in)	1/4	3/8	3/8	3/8	1/2
	al Anchor Dia. (in) w Anchor Body)	1/4	1/4	1/4	3/8	3/8
Nomi	inal Embedment, hoom (in)	1-5/8	1-5/8	2-1/2	2	2
Minim	um Edge Distance, cmin (in)	1-1/2	1-1/2	1-1/2	1-1/2	1-1/2
	1-1/2	0.77	0.77	0.64	0.74	0.74
<b>∞</b>	1-3/4	0.83	0.83	0.67	0.79	0.79
ches Ches	2	0.88	0.88	0.71	0.84	0.84
Ē	2-1/4	0.94	0.94	0.75	0.89	0.89
ance	2-1/2	1.00	1.00	0.78	0.95	0.95
Dist	2-3/4	1.00	1.00	0.82	1.00	1.00
Edge Distance (inches)	3	1.00	1.00	0.86	1.00	1.00
ш	3-1/2	1.00	1.00	0.93	1.00	1.00
	4	1.00	1.00	1.00	1.00	1.00

## **Edge Distance Reduction Factors - Shear (Fvc)**

Nominal A	nchor Size (in)	1/4	3/8	3/8	3/8	1/2
Nominal A (Screw A	nchor Dia. (in) Anchor Body)	1/4	1/4	1/4	3/8	3/8
Nominal Emb	edment, h <sub>nom</sub> (in)	1-5/8	1-5/8	2-1/2	2	2
	Edge Distance, nin (in)	1-1/2	1-1/2	1-1/2	1-1/2	1-1/2
	1-1/2	0.68	0.66	0.70	0.61	0.47
(88)	1-3/4	0.79	0.77	0.82	0.72	0.55
Distance (inches)	2	0.90	0.88	0.93	0.82	0.63
) eg	2-1/4	1.00	0.99	1.00	0.92	0.70
stan	2-1/2	1.00	1.00	1.00	1.00	0.78
e Di	2-3/4	1.00	1.00	1.00	1.00	0.86
Edge	3	1.00	1.00	1.00	1.00	0.94
	3-1/4	1.00	1.00	1.00	1.00	1.00

Spacing Reduction Factors - Tension ( $F_{\rm NS}$ )

Nomina	al Anchor Size (in)	1/4	3/8	3/8	3/8	1/2
	Anchor Diameter (in) w Anchor Body)	(in) 1/4 1/		1/4	3/8	3/8
Nominal E	Embedment, hoom (in)	1-5/8	1-5/8	2-1/2	2	2
Minimur	m Spacing, smin (in)	1-1/2	1-1/2	1-1/2	2	2
	1-1/2	0.73	0.73	0.66	-	-
	1-3/4	0.77	0.77	0.68	-	-
	2	0.80	0.80	0.70	0.77	0.77
les)	2-1/4	0.83	0.83	0.72	0.80	0.80
ii c	2-1/2	0.86	0.86	0.74	0.83	0.83
Spacing Distance (inches)	2-3/4	0.89	0.89	0.76	0.86	0.86
tan	3	0.92	0.92	0.78	0.89	0.89
i Di	3-1/2	0.99	0.99	0.82	0.94	0.94
cinç	4	1.00	1.00	0.86	1.00	1.00
Spa	4-1/2	1.00	1.00	0.90	1.00	1.00
	5	1.00	1.00	0.94	1.00	1.00
	5-1/2	1.00	1.00	0.97	1.00	1.00
	6	1.00	1.00	1.00	1.00	1.00

**Spacing Reduction Factors - Shear (Fvs)** 

Nomina	al Anchor Size (in)	1/4	3/8	3/8	3/8	1/2	
Nominal / (Scre	Anchor Diameter (in) w Anchor Body)	1/4	1/4	1/4	3/8	3/8	
Nominal I	Embedment, hnom (in)	1-5/8	1-5/8	2-1/2	2	2	
Minimu	m Spacing, smin (in)	1-1/2	1-1/2	1-1/2	2	2	
	1-1/2	0.61	0.61	0.62	-	-	
	1-3/4	0.63	0.63	0.64	-	-	
	2	0.65	0.65	0.66	0.64	0.60	
	2-1/4	0.67	0.66	0.68	0.65	0.62	
	2-1/2	0.69	0.68	0.69	0.67	0.63	
	2-3/4	0.71	0.70	0.71	0.69	0.64	
(89)	3	0.73	0.72	0.73	0.70	0.66	
Spacing Distance (inches)	3-1/2	0.76	0.76	0.77	0.74	0.68	
99	4	0.80	0.79	0.81	0.77	0.71	
stan	4-1/2	0.84	0.83	0.85	0.81	0.73	
ă	5	0.88	0.87	0.89	0.84	0.76	
Cin (Cin (Cin (Cin (Cin (Cin (Cin (Cin (	5-1/2	0.91	0.90	0.93	0.88	0.79	
Spa	6	0.95	0.94	0.97	0.91	0.81	
	6-1/2	0.99	0.98	1.00	0.94	0.84	
	7	1.00	1.00	1.00	0.98	0.86	
	7-1/2	1.00	1.00	1.00	1.00	0.89	
	8	1.00	1.00	1.00	1.00	0.92	
	9	1.00	1.00	1.00	1.00	0.97	
	10	1.00	1.00	1.00	1.00	1.00	



#### STRENGTH DESIGN INFORMATION

# Tension and Shear Design Information for Hangermate+ Anchor is in Concrete<sup>1,2,9,11,12</sup>



									Vants
Design Characteristic	Notation	Units		Nominal Anchor Size (inch)					
-			1/4		/8	3/8		3/8	1/2
Anchor category	1, 2 or 3	-	1		1	1		1	1
Coupler thread size (UNC)	-		1/4- 20	3/8-16		3/8-16		3/8-16	1/2-13
Coupler head style	-		Internal Thread	Internal Thread		External Thread		Internal Thread	Internal Thread
Nominal anchor diameter	da	in. (mm)	0.250		250	0.250 (6.4)		0.375	0.375
(screw anchor body)		in.	(6.4) 1-5/8	1-5/8	2-1/2	1-5/8	2-1/2	(9.5)	(9.5)
Minimum nominal embedment depth⁴	h <sub>nom</sub>	(mm)	(41)	(41)	(64)	(41)	(64)	(51)	(51)
Effective embedment	h <sub>ef</sub>	in. (mm)	1.20 (30)	1.20 (30)	1.94 (49)	1.20 (30)	1.94 (49)	1.33 (33)	1.33 (33)
Steel St	rength in Te	nsion (ACI 3	318-19 17.6.1, ACI		_ ` /	. ,	. ,	(/	(,
Steel strength in tension	Nsa	lb ((A))	4,535		535	4,5		8,730	8,730
Dadustian feature for atom attack	φ	(kN)	(20.2)	(20	0.2)	(20	).2)	(38.8)	(38.8)
Reduction factor for steel strength <sup>3,4</sup>		h in Tanala	- /ACI 210 10 17 6	ACI 240	14 17 4			(0)	
Concrete Brea	kout strengt		<b>n (ACI 318-19 17.6.</b> 4.3	<b>2, ACI 318</b>	6.1	4.3	6.1	5.0	5.0
Critical edge distance (uncracked concrete only)	Cac	in. (mm)	(110)	(110)	(156)	(110)	(156)	(127)	(127)
Effectiveness factor for uncracked concrete	Kuncr	-	27	27	24	27	24	30	30
Effectiveness factor for cracked concrete	Kcr	-	17	1	7	1	7	17	17
Modification factor for cracked and uncracked concrete⁵	$\Psi_{c,N}$	-				1.	.0		
Reduction factor for concrete breakout strength <sup>3</sup>	φ	-				0.0	 65		
Pullout Strength in Ter		eismic App	lications) (ACI 318-	19 17.6.3	ACI 318-			B-11 D.5.3)	
Characteristic pullout strength, uncracked concrete (2,500 psi) <sup>6,9</sup>	N <sub>p,uncr</sub>	lb (kN)	See Note 7	See I	Note 7	See Note 7		See Note 7	See Note 7
Characteristic pullout strength, cracked concrete (2,500 psi) <sup>6,9</sup>	N <sub>p,cr</sub>	lb (kN)	765 (3.4)	765 (3.4)	1,415 (6.3)	765 (3.4)	1,415 (6.3)	See Note 7	See Note 7
Reduction factor for pullout strength <sup>3</sup>	φ	-	(- /	(- /	(/	0.65			
Pullout Strength in Tens	ion for Seis	mic Applica	tions (ACI 318-19 1	7.10.3, A	CI 318-14	17.2.3.3 0	r ACI 318	3-11 D.3.3.3)	
Characteristic pullout strength, seismic (2,500 psi) <sup>6,8,9</sup>	N <sub>p,eq</sub>	lb (kN)	360 (1.6)	360 (1.6)	1,170 (5.2)	360 (1.6)	1,170 (5.2)	900 (4.0)	900 (4.0)
Reduction factor for pullout strength <sup>3</sup>	φ	-	,		, ,	0.0			
Steel S	trength in S	hear (ACI 3	18-19 17.7.1, ACI 3	18-14 17.	5.1 or ACI	318-11 D	.6.1)		
Steel strength in shear <sup>10</sup>	\/	lb	800	1,3	360	1,3	860	1,295	1,295
	V <sub>sa</sub>	(kN)	(3.6)	(6	.1)		.1)	(5.8)	(5.8)
Reduction factor for steel strength <sup>3,4</sup>	φ	-				0.0			
Steel Strength in Shea	r For Seism		<del>. `                                     </del>	<del></del>					1
Steel strength in shear <sup>10</sup>	Vsa,eq	lb (kN)	600 (2.7)	695 (3.1)		695 (3.1)		800 (3.6)	800 (3.6)
Reduction factor for steel strength <sup>3,4</sup>	$\phi$	-				0.0	60		
Concrete Brea	akout Streng	th in Shear	(ACI 318-19 17.7.2	, ACI 318-	14 17.5.2	or ACI 31	8-11 D.6.	2)	
Load bearing length of anchor	le	in. (mm)	1.20 (30)	1.20 (30)	1.94 (49)	1.20 (30)	1.94 (49)	1.33 (33)	1.33 (33)
Reduction factor for concrete breakout strength <sup>3,4</sup>	φ	-				0.	70		
Pryout :	Strength in S	Shear (ACI 3	318-19 17.7.3, ACI 3	18-14 17	.5.3 or AC	I 318-11 [	0.6.3)		
Coefficient for pryout strength	K <sub>cp</sub>	-	1	1	1	1	1	1	1
Reduction factor for pryout strength <sup>3,4</sup>	φ	-	İ	•		0.	 70		
For Clinian OF Amore 1 Inc. C 004 N/mm² 1 Mile 1	1	0.0044141							

For SI: 1 inch = 25.4 mm; 1 ksi = 6.894 N/mm²; 1 ft-lb = 1.356 N-m; 1 lb = 0.0044 kN.

- 1. The data in this table is intended to be used with the design provisions of ACI 318 (-19 or -14) Chapter 17 or ACI 318-11 Appendix D, as applicable; for anchors resisting seismic load combinations the additional requirements of ACI 318-14 17.2.3 or ACI 318-11 D.3.3, as applicable, shall apply.
- 2. Installation must comply with manufacturer's published installation instructions and details.
- 3. The strength reduction factor applies when the load combinations from the IBC or ACI 318 are used and the requirements of ACI 318-19 17.5.3, ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable, are met. If the load combinations of ACI 318-11 Appendix C are used, the appropriate strength reduction factor must be determined in accordance with ACI 318-11 D.4.4.
- 4. The anchors are considered a brittle steel elements as defined by ACI 318 (-19 or -14) 2.3 or ACI 318-11 D.1.
- 5. Select the appropriate effectiveness factor for cracked concrete ( $k_{cr}$ ) or uncracked concrete ( $k_{uncr}$ ) and use  $\psi_{c,N} = 1.0$ .
- 6. The characteristic pullout strength for concrete compressive strengths greater than 2,500 psi for 1/4-inch-diameter anchors (screw anchor body diameter) may be increased by multiplying the value in the table by (f'c / 2,500)<sup>n3</sup> for psi or (f'c / 17.2)<sup>n3</sup> for MPa. The characteristic pullout strength for concrete compressive strengths greater than 2,500 psi for 3/8-inch-diameter anchors (screw anchor body diameter) may be increased by multiplying the value in the table by (f'c / 2,500)<sup>n3</sup> for psi or (f'c / 17.2)<sup>n3</sup> for MPa.
- 7. Pullout strength does not control design of indicated anchors and does not need to be calculated for indicated anchor size and embedment.
- 8. Reported values for characteristic pullout strength in tension for seismic applications are based on test results per ACI 355.2, Section 9.5.
- 9. Reported values for steel strength in shear are based on test results per ACl 355.2, Section 9.4 and must be used for design in lieu of the calculated results using equation 17.7.1.2b in ACl 318-19, 17.5.1.2b in ACl 318-14 or equation D-29 in ACl 318-11 D.6.1.2.
- 10. Reported values for steel strength in shear are for seismic applications and based on tests in accordance with ACI 355.2, Section 9.6.
- 11. Anchors are permitted to be used in lightweight concrete in provided the modification factor  $\lambda_a$  equal to 0.8 $\lambda$  is applied to all values of √f'c affecting N<sub>n</sub>.
- 12. Hangermate+ shear values are for threaded rod or steel inserts with and ultimate strength, F<sub>u</sub> ≥ 125 ksi; threaded rod or steel inserts with an F<sub>u</sub> less than 125 ksi are allowed provided the steel strength shear values are multiplied by the ratio of F<sub>u</sub> (ksi) of the steel insert and 125 ksi.



# Tension and Shear Design Information for Hangermate+ Anchor in the Soffit (Through the Underside) of Concrete-Filled Steel Deck Assemblies<sup>1,2,3,4,5,6,9</sup>



Metation	Heito	Nominal Anchor Size (inch)								
Notation	UIIIIS	1/4	3/8		3/8		3/8	1/2		
1, 2 or 3	-	1	1		1		1	1		
-	in.	1/4-20	3/8-16		3/8-16		3/8-16	1/2-13		
-	-	Internal Thread	Internal	Thread	External Thread		Internal Thread	Internal Thread		
da	in. (mm)	0.250 (6.4)			0.250 (6.4)		0.375 (9.5)	0.375 (9.5)		
h <sub>nom</sub>	in. (mm)	1-5/8 (41)	1-5/8 (41)	2-1/2 (64)	(41)	(64)	2 (51)	2 (51)		
h <sub>ef</sub>	in. (mm)	1.20 (30)	1.20 (30)	1.94 (49)	1.20 (30)	1.94 (49)	1.33 (33)	1.33 (33)		
te+ Anchors	Installed in	to Minimum 3-7/8-	inch-wide	Deck Flu	te (See Fig	jure 6A)				
h <sub>min,deck,total</sub>	lb (kN)	5-1/2 (140)					5-1/2 (140)	5-1/2 (140)		
N <sub>p,deck,uncr</sub>	lb (kN)	1,430 (6.4)	1,430 (6.4)	2,555 (11.4)	1,430 (6.4)	2,555 (11.4)	2,275 (10.1)	2,275 (10.1)		
N <sub>p,deck,cr</sub>	lb (kN)	615 (2.7)	615 (2.7)	1,115 (5.0)	615 (2.7)	1,115 (5.0)	1,290 (5.1)	1,290 (5.1)		
N <sub>p,deck,eq</sub>	lb (kN)	290 (1.3)	290 (1.3)	920 (4.1)	290 (1.3)	920 (4.1)	890 (4.0)	890 (4.0)		
$\phi$	-	0.65								
V <sub>sa,deck</sub>	lb (kN)	1,205 (5.4)			1,205 (5.4)		1,360 (6.0)	1,360 (6.0)		
Vsa,deck,eq	lb (kN)	615 (2.7)			615 (2.7)		965 (4.3)	965 (4.3)		
$\phi$	-				0.6	60				
te+ Anchors	Installed in	to Minimum 1-3/4-	inch-wide	Deck Flu	te (See Fig	jure 6B)				
h <sub>min,deck,total</sub>	lb (kN)	4 (102)					4 (102)	4 (102)		
N <sub>p,deck,uncr</sub>	lb (kN)	1,430 (6.4)	1,430 (6.4)	2,075 (9.2)	1,430 (6.4)	2,075 (9.2)	1,440 (6.4)	1,440 (6.4)		
N <sub>p,deck,cr</sub>	lb (kN)	615 (2.7)	615 (2.7)	910 (4.0)	615 (2.7)	910 (4.0)	815 (3.6)	815 (3.6)		
N <sub>p,deck,eq</sub>	lb (kN)	290 (1.3)	290 (1.3)	750 (3.3)	290 (1.3)	750 (3.3)	565 (2.5)	565 (2.5)		
rength <sup>8</sup> $\phi$ -			0.65							
Vsa,deck	lb (kN)	815 (3.6)	815 (3.6)		815 (3.6)		1,110 (4.9)	1,110 (4.9)		
Vsa,deck,eq	lb (kN)	415 (1.8)	4 <sup>-</sup> (1	15 .8)	415 (1.8)		790 (3.5)	790 (3.5)		
φ	-				0.6	60				
	da hnom hef te+ Anchors hmin,deck,total Np,deck,cr Np,deck,eq  Vsa,deck Vsa,deck,eq  Anchors hmin,deck,total Np,deck,eq  Vsa,deck,eq  Vsa,deck,eq  Vsa,deck,eq  Vsa,deck,uncr Np,deck,uncr Np,deck,uncr Np,deck,uncr Np,deck,cr Np,deck,eq  Vsa,deck	1, 2 or 3 - in.  da in. (mm) hnom in. (mm) her in. (mm) her in. (mm) her in. (mm) hom,deck,total lb (kN) Np,deck,uncr lb (kN) Np,deck,cr lb (kN)  Np,deck,eq lb (kN)  Vsa,deck lb (kN)  Vsa,deck lb (kN)  Np,deck,eq lb (kN)  Vsa,deck lb (kN)  Vsa,deck lb (kN)  Np,deck,eq lb (kN)  Vsa,deck lb (kN)  Np,deck,eq lb (kN)  Vsa,deck lb (kN)  Np,deck,eq lb (kN)  Np,deck,total lb (kN)  Np,deck,total lb (kN)  Np,deck,uncr lb (kN)  Np,deck,eq lb (kN)  Np,deck,eq lb (kN)  Np,deck,eq lb (kN)  Vsa,deck lb (kN)	1/4	1, 2 or 3 - 1	1/4   3/8   1, 2 or 3   -   1   1   1   1   1   1   1   1   1	1,2 or 3	1,2 or 3 - 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1,2 or 3		

For SI: 1 inch = 25.4 mm, 1 ft-lb = 1.356 N-m, 1 psi = 0.0069 N/mm<sup>2</sup> (MPa).

- 1. Installation must comply with manufacturer's published installation instructions and details.
- 2. Values for No,deck and No,deck, are for sand-lightweight concrete (f'c, min = 3,000 psi) and additional lightweight concrete reduction factors need not be applied. In addition, evaluation for the concrete breakout capacity in accordance with ACI 318-19 17.6.2, ACI 318-14 17.4.2 or ACI 318 D.5.2, as applicable, is not required for anchors installed in the deck soffit (through underside)
- 3. Values for  $N_{p,deck,eq}$  are applicable for seismic loading.
- 4. The characteristic pullout strength for concrete compressive strengths greater than 3,000 psi for 1/4-inch-diameter anchors (screw anchor body diameter) may be increased by multiplying the value in the table by (f'c / 3,000)<sup>a3</sup> for psi or (f'c / 17.2)<sup>a3</sup> for MPa. The characteristic pullout strength for concrete compressive strengths greater than 3,000 psi for 3/8-inch-diameter anchors (screw anchor body diameter) may be increased by multiplying the value in the table by (f'c / 3,000)<sup>a3</sup> for psi or (f'c / 17.2)<sup>a3</sup> for MPa. For all design cases \( \frac{\psi}{2} \) = 1.0.
- 5. Shear loads for anchors installed through steel deck into concrete may be applied in any direction.
- 6. Values of V<sub>sa,deck,eq</sub> are for sand-lightweight concrete and additional lightweight concrete reduction factors need not be applied. In addition, evaluation for the concrete breakout capacity in accordance with ACl 318-19 17.7.2, ACl 318-14 17.5.2 or ACl 318-11 D.6.2, as applicable, and the pryout capacity in accordance with ACl 318-19 17.7.2, ACl 318-14 17.5.3 or ACl 318-11 D.6.3, as applicable, are not required for anchors installed in the soffit (through underside).
- 7. The minimum concrete member thickness, hmin,deck,total, is the minimum overall thickness of the concrete-filled steel deck (depth and topping thickness).
- 8. All values of  $\phi$  were determined from the load combinations of IBC Section 1605.2, 318 (-19 or -14) Section 5.3 or ACl 318 Section 9.2. If the load combinations of ACl 318 Appendix C are used, then the appropriate value of  $\phi$  must be determined in accordance with ACl 318-11 D.4.4.
- 9. Hangermate+ shear values are for threaded rod or steel inserts with and ultimate strength,  $F_u \ge 125$  ksi; threaded rod or steel inserts with an  $F_u$  less than 125 ksi are allowed provided the steel strength shear values are multiplied by the ratio of  $F_u$  (ksi) of the steel insert and 125 ksi.



# **DESIGN STRENGTH TABLES (SD)**

## Tension and Shear Design Strength Cracked Concrete 1,2,3,4,5,6,7,8



Namin	al Anghay Di				Minimum Concrete Compressive Strength										
Nomina			Nominal Embed.	Nominal Effective Embed.		f'c = 2,500 psi f'c		f'c = 3,000 psi		f'c = 4,000 psi		f'c = 6,000 psi		000 psi	
Coupler Thread Size (UNC)	Coupler Head Style	Screw Anchor Body	Depth hnom (in.)	nom	Depth hef (in.)	$\phi$ Nn Tension (lbs.)	<b>∲Vn</b> Shear (lbs.)	ØNn Tension (lbs.)	<b>ΦVn</b> Shear (lbs.)	ψNn Tension (lbs.)	<b>∲Vn</b> Shear (lbs.)	ØNn Tension (lbs.)	<b>ØVn</b> Shear (lbs.)	ØNn Tension (lbs.)	ØVn Shear (lbs.)
1/4 - 20	Internal Thread	1/4	1-5/8	1.20	495	515	525	515	575	515	645	515	705	515	
3/8 - 16	Internal	1/4	1-5/8	1.20	495	780	525	815	575	815	645	815	705	815	
3/0 - 10	Thread	1/4	2-1/2	1.94	920	815	970	815	1,060	815	1,195	815	1,305	815	
3/8 - 16	External	1/4	1-5/8	1.20	495	780	525	815	575	815	645	815	705	815	
3/0 - 10	Thread	1/4	2-1/2	1.94	920	815	970	815	1,060	815	1,195	815	1,305	815	
3/8 - 16	Internal Thread	3/8	2	1.33	845	775	930	775	1,070	775	1,315	775	1,515	775	
1/2 - 13	Internal Thread	3/8	2	1.33	845	915	930	1,000	1,070	1,140	1,315	1,140	1,515	1,140	
- Anchor	Pullout/Pryout	Strength Con	trols 🔲 - Con	icrete Breakou	it Strength Co	ontrols 🔳 - S	iteel Strength	Controls							

## Tension and Shear Design Strength Uncracked Concrete<sup>1,2,3,4,5,6,7</sup>



Namin	al Amehov Di			Minimum Concrete Compressive Strength										
Nomina	Nominal Anchor Diameter		Nominal Embed.	Effective Embed.	f'c = 2,500 psi		f'c = 3,000 psi		f'c = 4,000 psi		f'c = 6,000 psi		f'c = 8,000 psi	
Coupler Thread Size (UNC)	Coupler Head Style	Screw Anchor Body	Depth h <sub>nom</sub> (in.)	Depth hef (in.)	ØNn Tension (lbs.)	<b>∲Vn</b> Shear (lbs.)	$\phi$ Nn Tension (lbs.)	ψVn Shear (lbs.)	ØNn Tension (lbs.)	<b>ØVn</b> Shear (lbs.)	ØNn Tension (lbs.)	ØVn Shear (lbs.)	$\phi$ Nn Tension (lbs.)	φVn Shear (lbs.)
1/4 - 20	Internal Thread	1/4	1-5/8	1.20	1,155	515	1,265	515	1,460	515	1,785	515	2,065	515
3/8 - 16	Internal	1/4	1-5/8	1.20	1,155	815	1,265	815	1,460	815	1,785	815	2,065	815
3/0 - 10	Thread	1/4	2-1/2	1.94	2,110	815	2,310	815	2,665	815	2,950	815	2,950	815
3/8 - 16	1.c External	1/4	1-5/8	1.20	1,155	815	1,265	815	1,460	815	1,785	815	2,065	815
3/0 - 10	Thread	1/4	2-1/2	1.94	2,110	815	2,310	815	2,665	815	2,950	815	2,950	815
3/8 - 16	Internal Thread	3/8	2	1.33	1,495	775	1,640	775	1,890	775	2,315	775	2,675	775
1/2 - 13	Internal Thread	3/8	2	1.33	1,495	1,140	1,640	1,140	1,890	1,140	2,315	1,140	2,675	1,140
- Anchor	Pullout/Pryout	Strength Con	trols 🔲 - Cor	icrete Breakou	t Strength Co	ontrols 🔳 - S	Steel Strength	Controls						

- Tabular values are provided for illustration and are applicable for single anchors installed in normal-weight concrete with minimum slab thickness, ha = hmin, and with the following conditions:
- $c_{a1}$  is greater than or equal to the critical edge distance,  $c_{ac}$  (table values based on  $c_{a1} = c_{ac}$ ).
- Ca2 is greater than or equal to 1.5 times Ca1.
- Calculations were performed according to ACI 318 (-19 or -14) Chapter 17. The load level corresponding to the controlling failure mode is listed. (e.g. For tension: steel, concrete breakout and pullout; For shear: steel, concrete breakout and pullout; For shear: steel, concrete breakout and private the capacities for concrete breakout strength in tension and private. strength in shear are calculated using the effective embedment values, her, for the selected anchors as noted in the design information tables. Please also reference the installation specifications for more information.
- Strength reduction factors (ø) were based on ACI 318 (-19 or -14) Section 5.3 for load combinations. Condition B is assumed.
- Tabular values are permitted for static loads only, seismic loading is not considered with these tables.
- For designs that include combined tension and shear, the interaction of tension and shear loads must be calculated in accordance with ACI 318 (-19 or -14) Chapter 17.
- Interpolation is not permitted to be used with the tabular values. For intermediate base material compressive strengths please see ACI 318 (-19 or -14) Chapter 17.
- For other design conditions including seismic considerations please see ACI 318 (-19 or -14) Chapter 17.
  For seismic design of anchors installed in regions designated as Seismic Design Categories C, D, E or F and in accordance with ACI 318, the tabulated tension design strengths in cracked concrete for concrete breakout and pullout must be multiplied by a factor of 0.75.
- Hangermate+ shear values are for threaded rod or steel inserts with an ultimate strength, F<sub>u</sub> ≥ 125 ksi, threaded rod or steel inserts with an F<sub>u</sub> less than 125 ksi are allowed provided the steel strength shear values are multiplied by the ratio of Fu (ksi) of the steel insert and 125 ksi.



#### Tension and Shear Design Strength of Steel Elements (Steel Strength)<sup>1,2,3,4</sup>

	Steel Elements - Threaded Rod											
Nominal Rod Diameter	ASTM A36 and AS	TM F1554 Grade 36	ASTM F155	64 Grade 55	ASTM A193 Grade B7 and ASTM F1554 Grade 105							
(in.)	φN <sub>sa,rod</sub> Tension (lbs.)	ψV <sub>sa,rod</sub> Shear (lbs.)	Shear Tension		<b>∲N</b> sa,rod <b>Tension</b> ( <b>lbs.</b> )	φV <sub>sa,rod</sub> Shear (lbs.)						
0.25	1,385	720	1,790	930	2,985	1,550						
0.375	3,370	1,750	4,360	2,265	7,265	3,775						
0.5	6,175	3,210	7,980	4,150	13,300	6,915						

#### - Steel Strength Controls

- Steel tensile design strength according to ACI 318 (-19 or -14) Ch. 17 and ACI 318-11 Appendix D, φNsa = φ Ase, N futa
- 2. The tabulated steel design strength in tension for the threaded rod must be checked against the design strength of the Hangermate+ steel, concrete breakout and pullout design strength to determine the controlling failure mode, the lowest load level controls.
- 3. Steel shear design strength according to ACI 318 (-19 or -14) Ch. 17 and ACI 318-11 Appendix D,  $\phi_{\text{Nsa}} = \phi \bullet 0.60 \bullet \text{Asa,V} \bullet \text{futa}$
- 4. The tabulated steel design strength in shear for the threaded rod must be checked against the design strength of the Hangermate+ steel, concrete breakout and pryout design strengths to determine the controlling failure mode, the lowest load level controls.

#### ORDERING INFORMATION

Catalog	Carrour Cima	Иоми	Bod Circ	Cooket Circ	Pack	Carton	Suggested A	1 Accessories			
Number	Screw Size	Hang	Rod Size	Socket Size	Qty.	Qty.	SDS+ Carbide Drill Bits	Hangermate+ Driver			
Hangermate+ Internal Thread (UNC)											
PFM2211100	1/4" x 1-5/8"	Vertical	1/4"	3/8"	25	125	DW5517, DW5417	PFM1491050			
PFM2211200	1/4" x 1-5/8"	Vertical	3/8"	1/2"	25	125	DW5517, DW5417	PFM1491100			
PFM2211250	1/4" x 2-1/2"	Vertical	3/8"	1/2"	25	125	DW5517, DW5417	PFM1491100			
PFM2211260	3/8" x 1-5/8"	Vertical	3/8"	1/2"	25	125	DW5527, DW5427	PFM1491100			
PFM2211270	3/8" x 2"	Vertical	3/8"	1/2"	25	125	DW5527, DW5427	PFM1491100			
PFM2211280	3/8" x 2"	Vertical	1/2"	11/16"	20	100	DW5527, DW5427	-			
Hangermate+ External Thread (UNC)											
PFM1421000	1/4" x 1-5/8"	Vertical	3/8"	1/2"	25	125	DW5517, DW5417	DWMT19052B			
PFM1421050	1/4" x 2-1/2"	Vertical	3/8"	1/2"	25	125	DW5517, DW5417	DWMT19052B			
PFM1421050						125	DW5517, DW5417	DWMT19052			

The published size includes the diameter and length of the anchor measured from under the head.

Shaded catalog numbers denote sizes which are less than the minimum standard anchor length for Strength Design.



## **Suggested Tools**

